

CALIFORNIA COASTAL COMMISSION

NORTH COAST DISTRICT OFFICE
1385 EIGHTH STREET, SUITE 130
ARCATA, CA 95521
VOICE (707) 826-8950
FAX (707) 826-8960



F8a

1-20-0360 (YATES)

NOVEMBER 6, 2020

EXHIBITS

Exhibit 1 – Regional Location

Exhibit 2 – Vicinity Map

Exhibit 3 – Flood Zone

Exhibit 4 – Site Plan

Exhibit 5 – 90% Plans

Exhibit 6 – Soil Study (excerpts)



DEL NORTE

Redwood NP

Stone Lagoon

Big Lagoon

Patrick's Pt.

Big Lagoon Rancheria

City of Trinidad

Trinidad Rancheria

City of McKinleyville

Blue Lake Rancheria

Arcata Bay

City of Arcata

Humboldt Bay

City of Eureka

HUMBOLDT

Table Bluff Reservation

Bear River Band - Rohnerville Rancheria

City of Fortuna

City of Ferndale

Eel R

Cape Mendocino

King Range NCA

MENDOCINO

Approximate Project Location

EXHIBIT NO. 1
CDP Application No.
1-20-0360 (Yates)
REGIONAL LOCATION

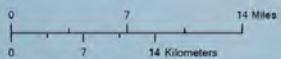




EXHIBIT NO. 2
CDP Application No.
1-20-0360 (Yates)
VICINITY MAP

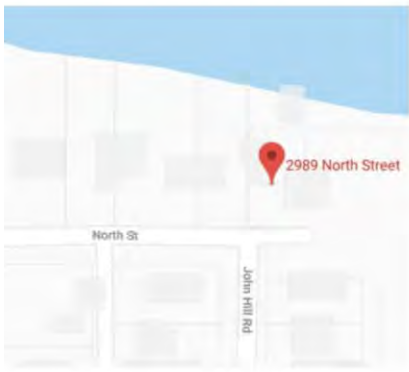
**100 Year
Flood
Zone (A)**

Subject Parcel



EXHIBIT NO. 3

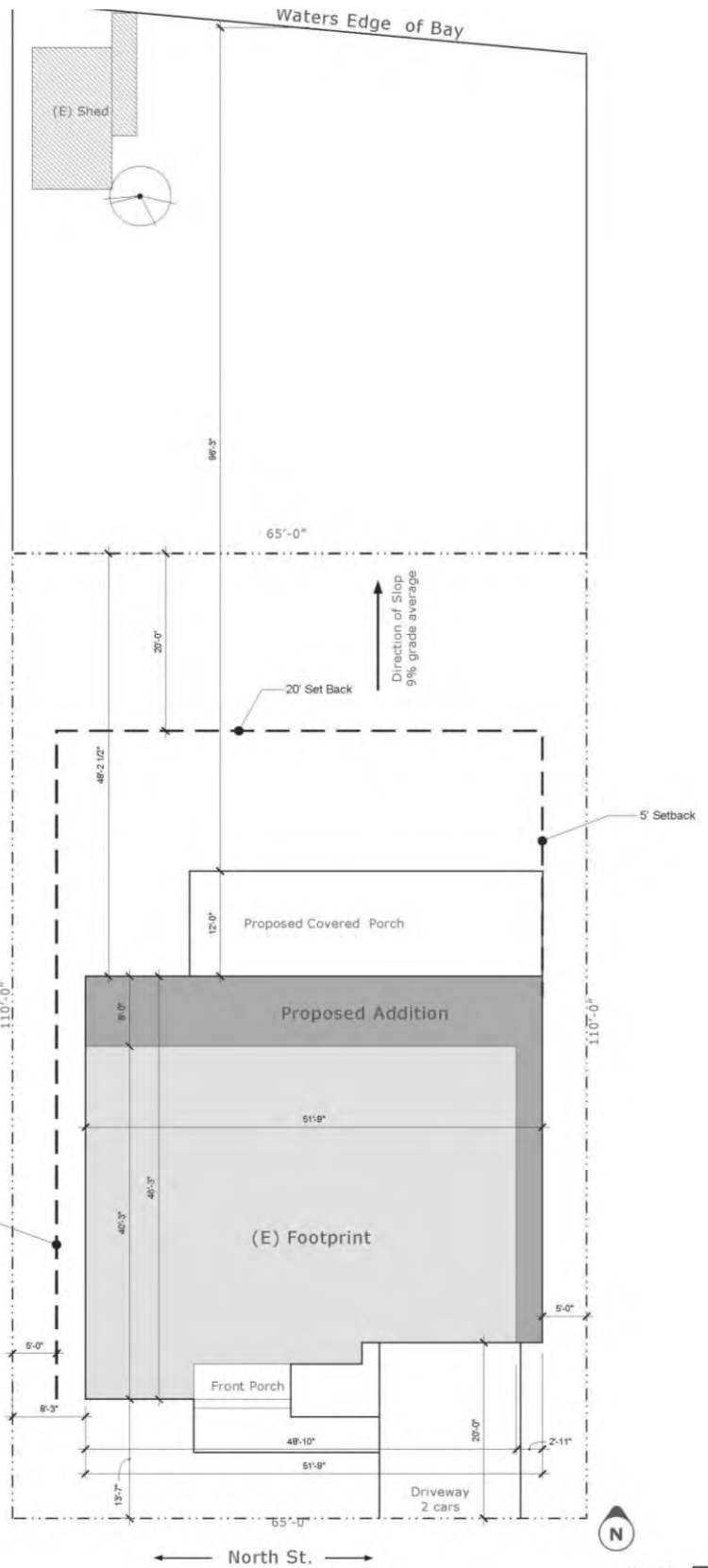
CDP Application No.
1-20-0360 (Yates)
FLOOD ZONE




Vicinity Map A
scale: NA 03



Humboldt County GIS Map A
scale: NA 02



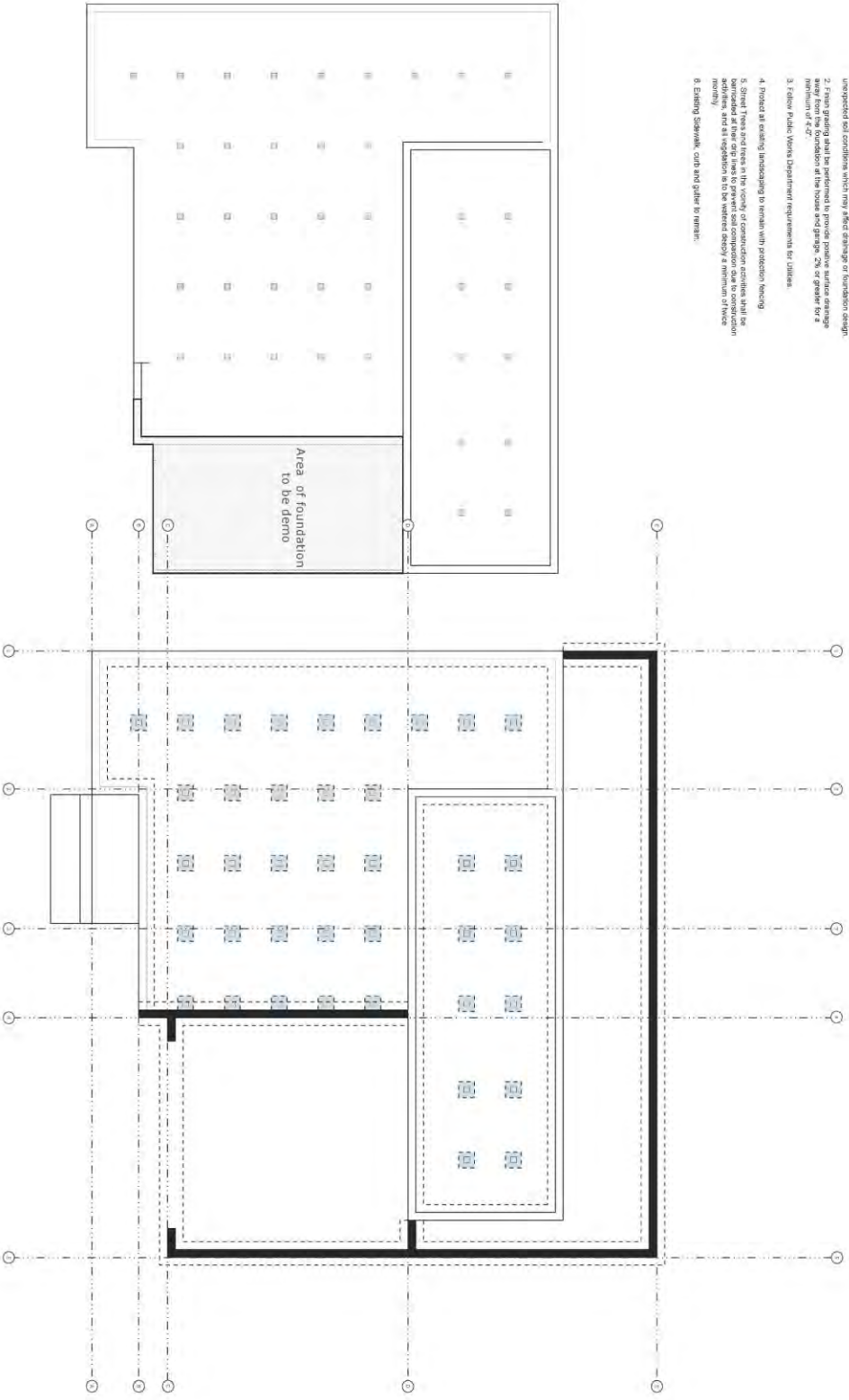
Site Legend

- Property line: - - - - -
- Setback Lines: - - - - -
- Existing Tree: 

Site Plan A

EXHIBIT NO. 4
CDP Application No.
1-20-0360 (Yates)
SITE PLAN

- Site Notes**
1. All Construction Work shall be done in accordance with the Engineer's specifications and shall be subject to the Engineer's inspection and approval. The Contractor shall be responsible for obtaining all necessary permits and for obtaining all necessary approvals from the appropriate authorities. The Contractor shall be responsible for obtaining all necessary approvals from the appropriate authorities.
 2. Erosion control measures shall be installed to prevent erosion and sedimentation. Erosion control measures shall be installed to prevent erosion and sedimentation.
 3. Follow Public Works Department requirements for Utilities.
 4. Protect all existing landscaping to remain with protection fencing.
 5. Street Trees and trees in the vicinity of construction activities shall be protected at their expense to prevent soil compaction due to construction activities. The Contractor shall be responsible for obtaining all necessary approvals from the appropriate authorities.
 6. Existing sidewalk, curb and gutter to remain.



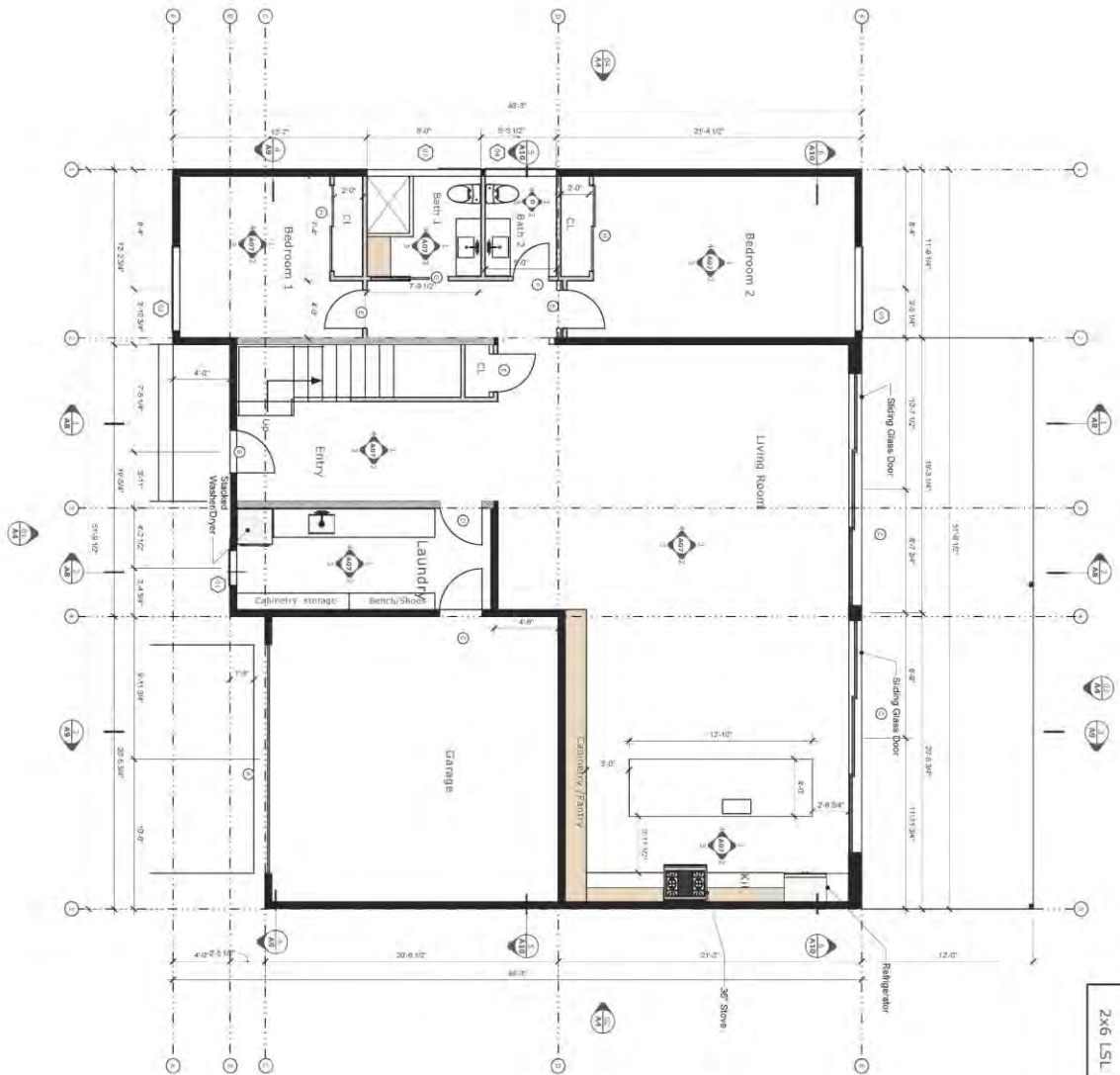
Existing foundation Plan A
Scale: 1/4" = 1'-0"

Proposed foundation Plan A
Scale: 1/4" = 1'-0"

EXHIBIT NO. 5

CDP Application No. 1-20-0360 (Yates)

90% PLANS (page 1 of 6)



Wall Types	
2x6	(Thick solid line)
2x4	(Medium solid line)
2x6 LSL	(Dashed line)

FIRST FLOOR PLAN
Scale: 1/8" = 1'-0"

03

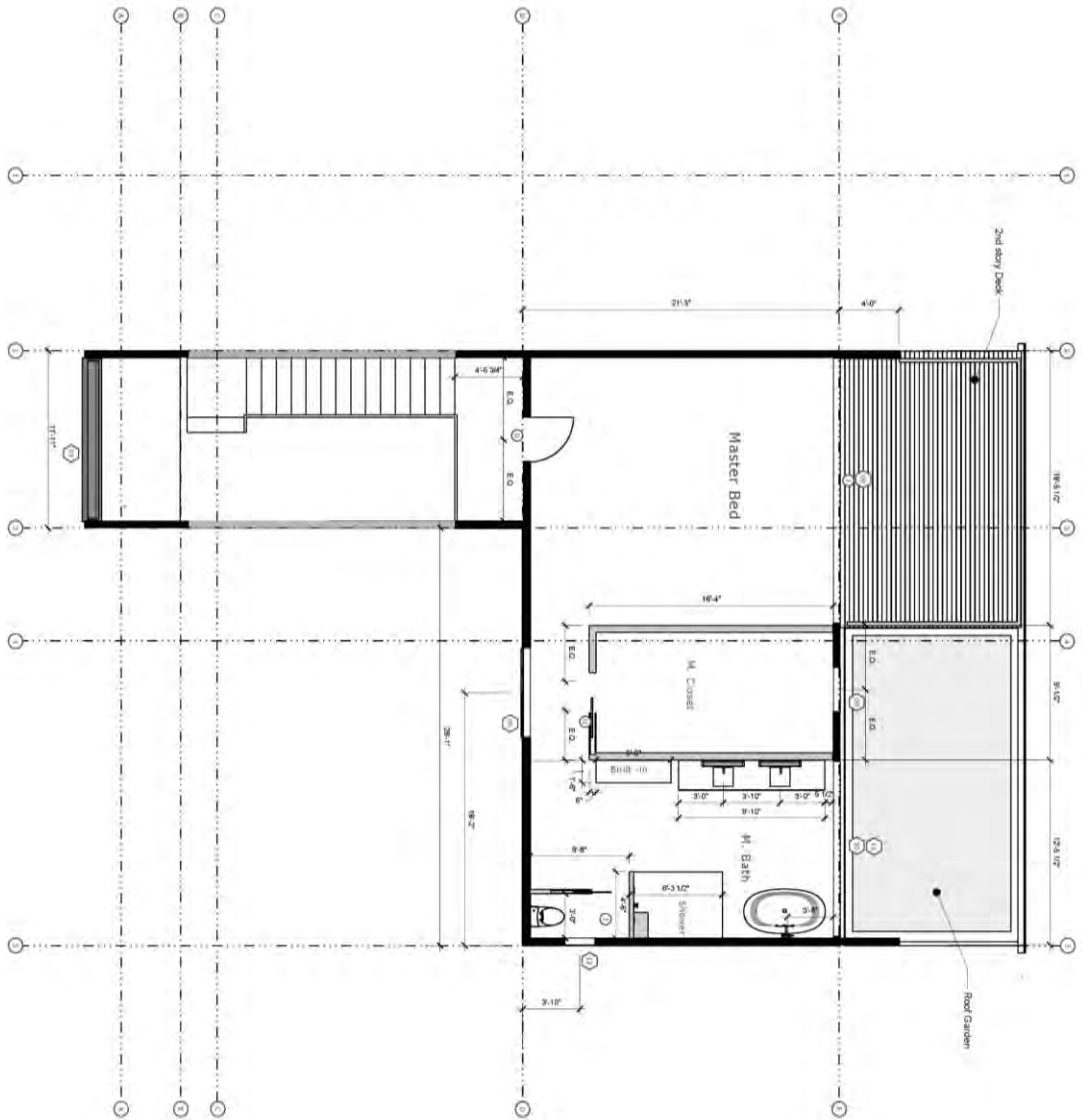
Sheet Description
**Proposed Floor Plan,
Window & Door Schedule**

CLIENT
Karvier Yates
2989 North St
Eureka, CA 95501

Design BY
Charlie Hatfield design
701 Duncumville Ct., Campbell, CA 95008
phone: 650-245-4734
email: charliehatfield@me.com

CHD
PROJECT NO.
25047 - 6/22/20

EXHIBIT NO. 5
CDP Application No.
1-20-0360 (Yates)
90% PLANS (page 2 of 6)



Second Floor Plan A
 04
 2024-11-21 9:00

04

a

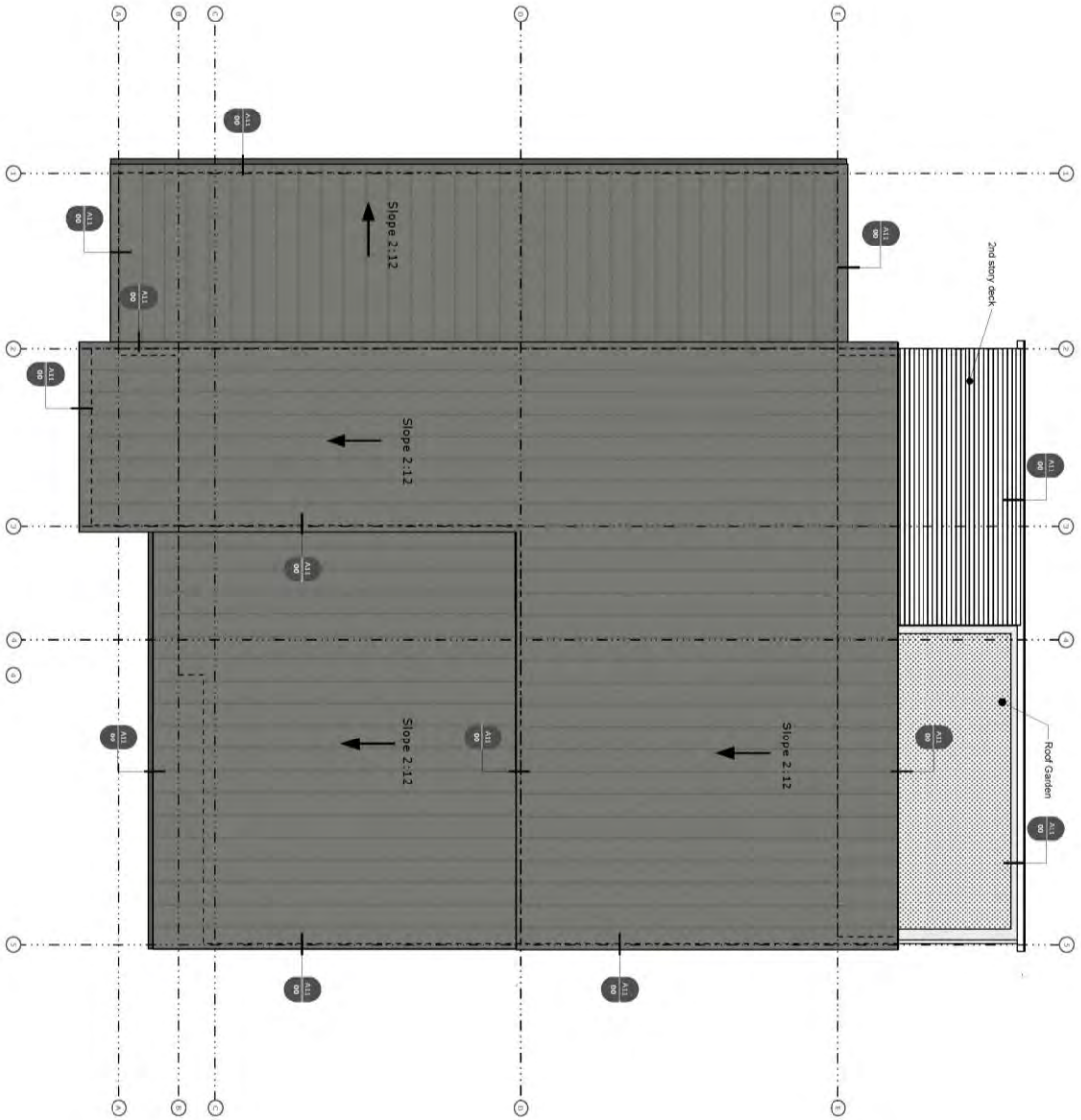
Sheet Description
Proposed Second Plan,

CLIENT
Karvier Yates
 2989 North St.
 Eureka, CA 95501

Design BY
Charlie Hatfield design
 701 Duncanville Ct., Campbell, CA 95008
 phone: 925-248-2744
 email: charliehatfield@mac.com

CHD
 PROJECT NO.
 2024-11-21
 ISSUE: 14/2/24

EXHIBIT NO. 5
 CDP Application No.
 1-20-0360 (Yates)
90% PLANS (page 3 of 6)



Roof Plan A
 scale: 3/4"=1'-0"

05

a

Sheet Description
Roof Plan

CLIENT:
Karvier Yates
 2889 North St.
 Eureka, CA 95501

Design BY
Charlie Hatfield design
 701 Duncansville Ct, Campbell, CA 95008
 phone: 650-248-4744
 email: charliehatfield@mac.com

CHD
 PROJECT NO.
 2015.1
 ISSUE - 6/23/20

EXHIBIT NO. 5
 CDP Application No.
 1-20-0360 (Yates)
90% PLANS (page 4 of 6)

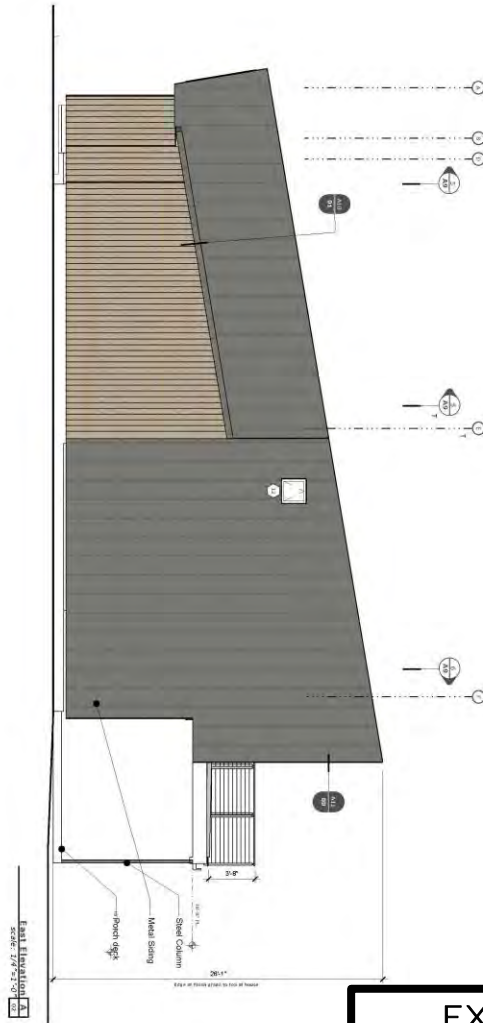
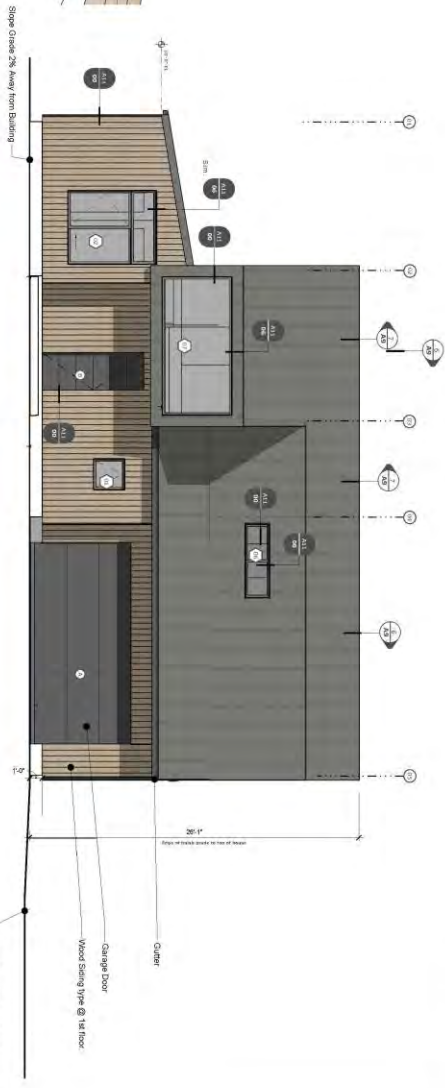
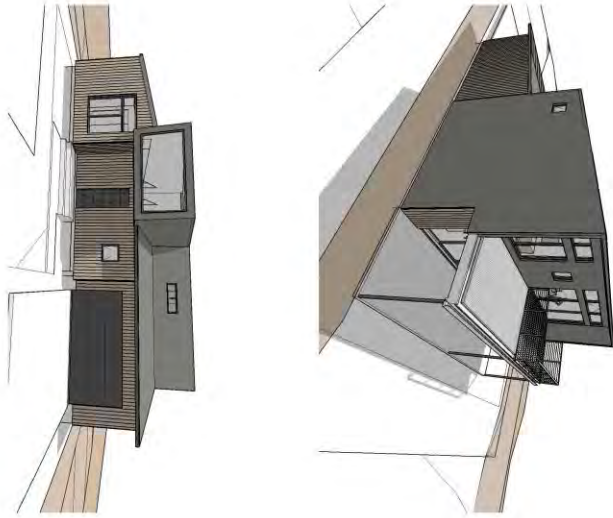
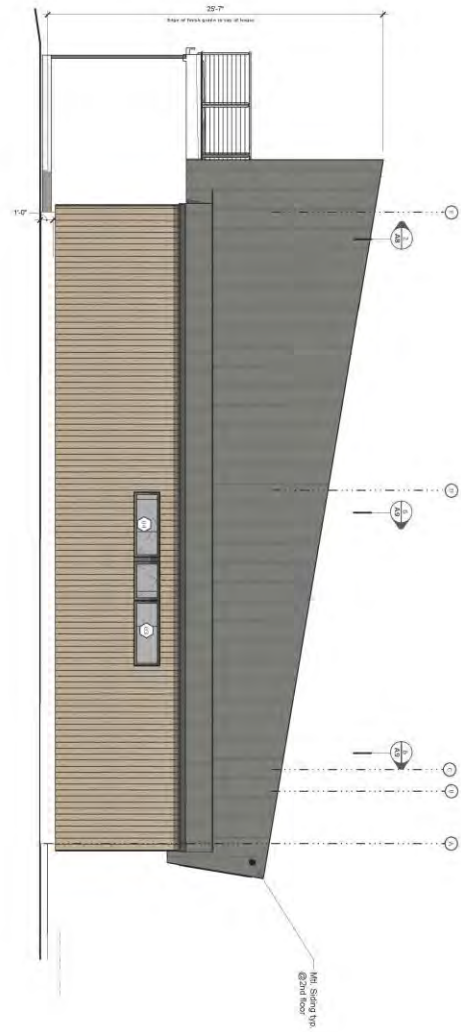


EXHIBIT NO. 5

**CDP Application No.
1-20-0360 (Yates)
90% PLANS (page 5 of 6)**



Slope Grade 2% Away from Building



07 a

Sheet Description
Elevations

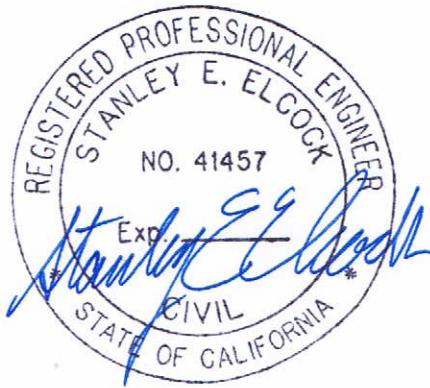
CLIENT:
Karvier Yates
2989 North St.
Eureka, CA 95501

Design BY
Charlie Hatfield
731 Dunsmuir Ct, Campbell
phone: 855-245-7144
email: charliehatfield@mac.com

EXHIBIT NO. 5
CDP Application No.
1-20-0360 (Yates)
90% PLANS (page 6 of 6)

SOIL STUDY
FOR YATES RESIDENCE
2989 North St.
APN 014-203-006
IN MYRTLETOWNE

PREPARED FOR:
Karvier Yates



PREPARED BY:
S.E.E. ENGINEERING

August 6, 2020

EXHIBIT NO. 6

CDP Application No.
1-20-0360 (Yates)
SOIL STUDY (page 1 of 6)

SOIL STUDY
FOR YATES RESIDENCE
2989 North St.
APN 014-203-006
IN MYRTLETOWNE

A soils investigation was conducted on August 4, 2020 on the site of a proposed residence. The site is located at 2989 North Street in Eureka, California.

The purpose of the study was to determine the types and extent of the materials to be encountered during construction with recommendations for their utilization and site development criteria for the proposed project, including incorporating the existing foundation into the new residence.

FIELD INVESTIGATION

S. E. E. Engineering field investigations were limited to reconnaissance of the project site and drilling and sampling. Two exploratory borings were advanced to a maximum depth of 7'-0" inches below ground surface in Boring No.1 and Boring No.2. Ground water was encountered at 79" in Boring No.1 and no ground water was encountered in Boring No.2. A total of 6 moisture samples were taken with the results as follows:

- Bore No.1 = 17.5% at 12 inch depth
- Bore No.1 = 8.0% at 42 inch depth
- Bore No.1 = 29.6% at 84 inch depth
- Bore No.2 = 10.6% at 27 inch depth
- Bore No.2 = 17.3% at 60 inch depth
- Bore No.2 = 24.0% at 84 inch depth

Samples were obtained and logged in accordance with the Unified Soil Classification system and were rated SW, SM, and OH at the bottom of Boring No.2.. The borings were advanced using a hand auger to a depth of 7 feet below ground surface.

EXHIBIT NO. 6

CDP Application No.
1-20-0360 (Yates)
SOIL STUDY (page 2 of 6)

SITE CONDITIONS AND MATERIAL PRESENT

The site is located at the north end of North Street in Eureka. The area has a current residence and associated grading. Specific descriptions of the soils encountered are presented in the attached boring logs.

Most of the material was moist to wet and firm in nature. The material graded from 0 to 54” of light brown to medium brown silty sand, 54” to 84” medium brown sand, at 84” bluish gray silty clay in the bottom of Boring No.2. Free water was encountered at 79” in Boring No.1. There was some mottling in Boring No.1.

The tests taken with the Pocket Penetrometer revealed a compressive strength of 4 tons/sq. ft. at about 15” below grade in Boring No.2.

The site is approximately 1.5 kilometers South of a trace of the Freshwater fault and is approximately 2 to 3 kilometers North of the Little Salmon Fault which is actually South and East of the site. The Freshwater Fault is a type A fault and the Little Salmon is considered a type B, based on the 2019 CBC Seismic Design category is “E”: Site Classification of “D” with site coefficients F_a and F_v equal to 1.0 and 1.5 respectively. We estimate a soil profile type S_d for the site. The calculated seismic response coefficient is -0.25.

The site is mapped in a “Co” area by the Humboldt County General Plan Geologic map (Humboldt County 1989). This is a generalized map and it indicates the site is in a relatively stable area with respect to land sliding, that it is composed of Pleistocene Non-Marine Formation.

The project site is mapped outside a potential tsunami inundation zone by Special Publication 115 (CDMG 1995), in which is based on a Tsunami Inundation Model Study of Eureka and Crescent City, California by Bernard, Mader, Curtis and Satake, 1994. According to locally authored tsunami safety map; it is also outside a potential tsunami inundation area (Patton and Dengler 2004).

EXHIBIT NO. 6

CDP Application No.
1-20-0360 (Yates)
SOIL STUDY (page 3 of 6)

GEOLOGY

The sediment in the area is mainly recent Alluvium deposits of Humboldt Bay to Quaternary Non-Marine terrace deposits of the Rohnerville formation. The sediments encountered are Pleistocene to recent (less than 11,000 years).

As stated earlier, the project is located between the Freshwater Fault and the Little Salmon Fault. These are all thrust faults and are considered active. Thrust faults are low angle faults (30 degrees to 40 degrees) that build up considerable horizontal stress before they fail and can generate a large event.

A fault zone is a zone of related faults that commonly are braided and sub-parallel, but may be braiding and divergent. A fault zone has significant width ranging from a few feet to several miles.

An active fault is defined by the Mining and Geology Board as one which has "Bad Surface Displacement within Holocene time (about the last 11,000 years before present)

The project area is subject to strong seismic motion as is a good part of Humboldt County, but the incidence of damage could be rare. Due to the sites location, it would be considered in a relatively stable area with respect to land slidings. Liquefaction could be a problem in the event of a relatively rare, major earthquake. For example, we are not aware of liquefaction or no seismic settlement occurring in the area as a result of the 1992 Petrolia earthquake, which had a maximum movement magnitude of approximately 7.

Due to its elevation and location the site is to be considered a low tsunami risk.

EXHIBIT NO. 6 |

CDP Application No.
1-20-0360 (Yates)
SOIL STUDY (page 4 of 6)

TSUNAMI RISK

Tsunami Risk may be indicated in the two publications cited under "Site Conditions" above. The December 26, 2004 tsunami in Sumatra resulted in west coast shoreline tsunami heights in Sumatra (approximately opposite the epicenter) on the order of 20 to 35 meters (66 to 115 feet), per the U. S. Geological Survey(USGS)webpage<http://walrus.wr.usgs.gov/tsunami/sumatra05/> as of January 18, 2006. The causative subduction zone geology in Sumatra is generally analogues to that existing at Humboldt County's off shore Cascadia subduction zone, which may indicate higher wave height potential than considered in previous publications. However, close to the elevation of the project site the event of a tsunami would be minimal to a slight possibility. Using the most current Tsunami Hazard Maps published by the Redwood Coast Tsunami Watch Group, the proposed site is in a low hazard zone and risk is considered speculative in a rare, extreme causative event.

LIQUEFACTION, CO-SEISMIC SETTLEMENT, LATERAL SPREADING

Relatively strong, prolonged earthquakes are the common source of dynamic loading causing liquefaction. During shaking, the soil structure tends to collapse, while pore ground water pressure build up in the soil until sheer strength is lost, and soil/ground water mixture temporarily act as liquid slurry until excess pore pressure dissipates.

Co-seismic settlement of moderately consolidate, sandy relatively cohesion loose soils can occur above or below ground water, and is characterized by soil densification resulting from dynamic loading of relatively loose or moderately consolidate granular soil, resulting in settlement of the ground surface.

Lateral spreading is considered a low risk at the subject site. For this report, risk of potentially liquefiable soils subject to co-seismic compaction is a decided possibility.

GENERAL SEISMIC DESIGN

The minimum standard for construction of the structure should be in accordance with the latest edition of the CBC for the most seismic areas.

EXHIBIT NO. 6

CDP Application No.
1-20-0360 (Yates)

SOIL STUDY (page 5 of 6)

CONCLUSIONS AND RECOMMENDATIONS

Based upon the results of the study it is our opinion that the residence remodel can be constructed as proposed. The potential for compressible soil is limited; liquefaction is possible and tsunami risk is minimal. No high plasticity soils strata were encountered or are generally anticipated under the geologic formation comprising the site and risk of adverse consequences to the structure from expansive soil is considered low.

It is recommended that the bottom of new footings be between 6” to 12” below the existing grade for a design bearing pressure of 2000 PSF.

Surface drainage should be directed away from the foundation using ditches and sloping the grade and piping gutter drains away from the structure.

There is no conflict with any Alquist-Priolo study zones.

LIMITATIONS OF REPORT

The analysis, conclusions, and recommendations contained in this report are based on the site conditions at the time of the investigation. We have assumed that the information obtained from our subsurface investigation is representative of the subsurface conditions throughout both sites.

If there is a substantial change in the scope of the proposed work or if during construction the subsurface conditions differ significantly, then SEE ENGINEERING should be contacted to re-evaluate the applicability of our conclusions and recommendations.

The conclusions and recommendations contained in this report are professional opinions based on current standards of professional practice. The recommendations are based on the assumption that design improvements will conform to their intent. Assumptions about other site characterist

EXHIBIT NO. 6

CDP Application No.
1-20-0360 (Yates)
SOIL STUDY (page 6 of 6)